



# TRANSPORTATION RESEARCH SYNTHESIS

Minnesota Department of Transportation  
Office of Policy Analysis, Research & Innovation  
Research Services Section  
(651) 366-3780  
[www.research.dot.state.mn.us](http://www.research.dot.state.mn.us)

TRS 1203

Published July 2012

## INNOVATIVE BRIDGE CONSTRUCTION FOR MINNESOTA LOCAL ROADS

### Introduction

Minnesota counties have been actively implementing new design and construction techniques for multiple projects in recent years. In June of 2011, the Local Road Research Board (LRRB) discussed and approved preparing a synthesis report to document several of these projects. The need statement was originally titled “Accelerated Bridge Construction”. The title was then broadened to “Innovative Bridge Construction for Minnesota Local Roads” when the MnDOT Research Services Section developed the contract.

Scanning tours have previously been conducted to investigate bridge design and construction methods used in other states. Staff from the MnDOT State Aid Office and the Bridge State Aid Unit, representatives from the Minnesota County Engineers Association, and FHWA have participated in the scans. Several of the technologies identified during these scans have since been applied to local bridge construction on Minnesota projects, and are included in this synthesis.

When the LRRB approved the synthesis project, specific example technologies were cited for inclusion such as the proposed Rock County project using geosynthetic-reinforced soils abutments, and precast inverted tee sections for slab spans. During development of the contract additional technologies were

identified and included. At the County Engineers Bridge Subcommittee meeting in January 2012, large culvert projects were added. The resulting six technologies addressed in this synthesis are:

- Geosynthetic-Reinforced Soil (GRS) Abutments – Rock County
- Precast Inverted Tee Slab Span Bridges – Scott and Chisago County
- Precast Box Beams and Sheet Pile Abutments – Blue Earth County
- MSE Wall with Single Line Pile Abutment – Steele County
- Crash Tested Open Metal Railing – Polk County
- Large Precast Box Culverts – Aitkin County, and Three-Sided Structures – Multiple Locations

Construction projects have been completed on the County system using all of these technologies except the inverted tee section and geosynthetic reinforced soil abutments. However, MnDOT has built multiple inverted tee bridges and the first county projects are in design. The GRS abutment project for Rock County is scheduled for 2012 construction.

In cooperation with the MnDOT, the LRRB, and the participant County Engineers, this report was prepared by Dan Dorgan and Christopher Werner of HDR Engineering. The input and cooperation of all those who contributed is greatly appreciated.

## **Organization of Synthesis Report**

The report is separated into individual discussions of each of the six subjects. A description of the technology, the benefits and limitations, cost information as available, and potential implementation actions where appropriate, are described.

The synthesis includes a summary of potential research topics or implementation actions to further innovate and accelerate bridge construction for local roads. These were gathered from the various engineers interviewed.

Additional photos or drawings are included in the Appendix and referenced according to the number, from 1 to 6, assigned to the innovation. The contents of this report are as follows:

Introduction.....	p. 1 – 2
Synthesis Process.....	p. 4
Innovation 1: GRS Abutments.....	p. 4 – 7
Innovation 2: Precast Inverted Tee Slab Span Bridges.....	p. 7 – 10
Innovation 3: Precast Box Beams and Sheet Pile Abutments.....	p. 10 – 14
Innovation 4: MSE Wall with Single Line Pile Abutment.....	p. 14 – 15
Innovation 5: Crash Tested Open Metal Railing.....	p. 15 – 18
Innovation 6: Large Precast Box Culverts and Three-Sided Structures.....	p. 18 – 21
Recommendations for Research or Implementation.....	p. 21 – 22
Opportunities for Federal Funding of Innovative Designs.....	p. 22 – 23
Innovative Bridge Techniques Summary.....	p. 24
Appendix.....	p. 25 – 40

## **Synthesis Process**

The author of the report attended the 2012 County Bridge Engineers Subcommittee meeting in January to discuss and confirm the projects within the synthesis. At that meeting, the following definition for Innovative Bridge Construction was accepted for purposes of the synthesis:

*Innovative bridge construction is the use of bridge structures or components not typically employed in Minnesota bridges on the local road system, which offer potential savings in construction cost and/or construction schedule. Technologies or methods that are transferred from practice in other states or developed within Minnesota are equally valued.*

Data was collected for use in the synthesis by interviewing County Engineers and staff, the MnDOT State Aid Bridge staff, FHWA Minnesota Division Bridge Engineer, and consulting engineers involved in the projects. Site visits were conducted for several of the technologies.

### **Innovation 1: Geosynthetic-Reinforced Soil (GRS) Abutments –Rock County (Bridge 67564)**

#### *Description*

The abutments for Rock County Bridge 67564 will be constructed using geosynthetic reinforced earth abutments. The bridge carries Rock County Road 55 over a regional railroad. The GRS system is composed of alternating layers of geosynthetic fabric with backfill in 4- to 8-inch layers. The fabric is polypropylene, provides the reinforcement for the system, and together with the soil layers transfers the horizontal load that would exert active pressure on the back face of traditional abutments back beyond the failure plane of the backfill. The GRS mass is stabilized internally by the interaction of the reinforcing fabric and backfill. The front facing of the abutment is typically gravity stacked 8-inch concrete blocks. The lower layers, where water may be anticipated, are constructed with solid concrete blocks to prevent water from accumulating and freezing. Upper layers where no water is anticipated are hollow.

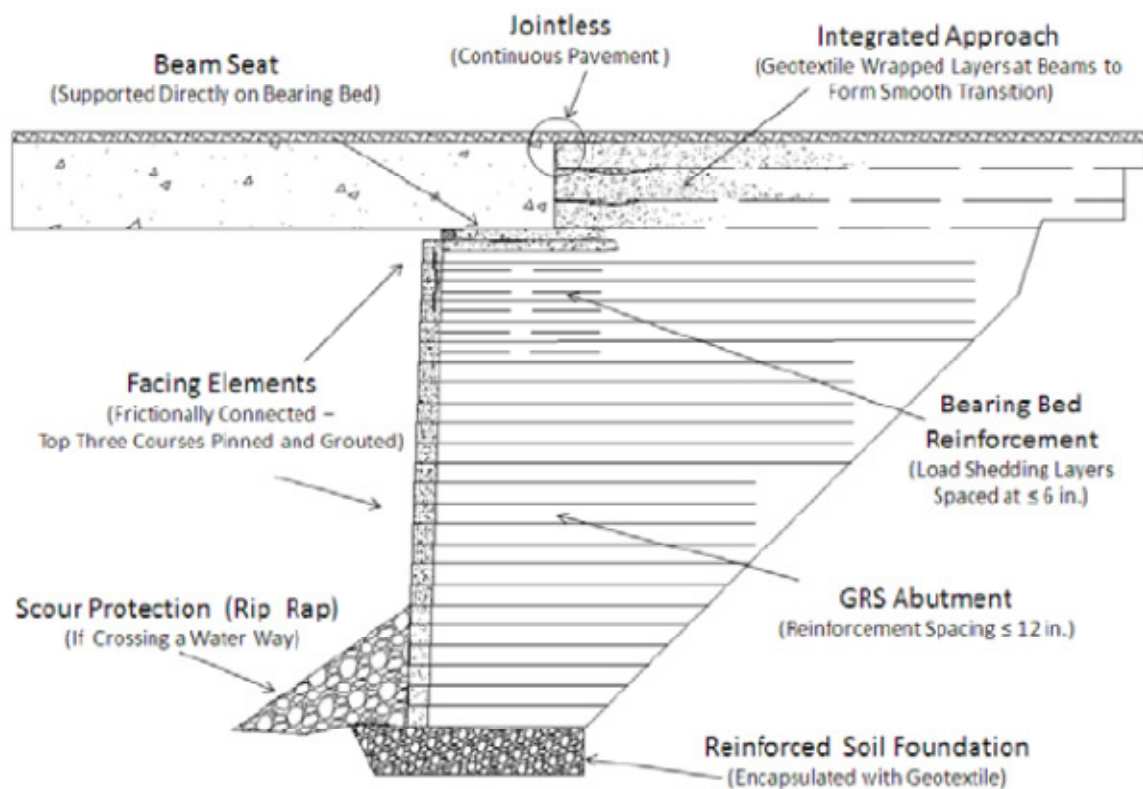


**1-1 Example of GRS Abutment (courtesy of FHWA)**

The Rock County project has received Federal Innovative Bridge Research and Deployment (IBRD) funding as a first demonstration of GRS abutments in Minnesota. According to the County Engineer, the availability of the IBRD funds significantly contributed to the decision to use this technology at this location. If the County were to have funded the project entirely from local sources, they likely would not have been able to fund a replacement at this time. The superstructure will be concrete precast adjacent box beams.

The Federal Highway Administration (FHWA) has developed a website with a sample design for GRS systems along with project information. A number of structures have been built in Defiance County, Ohio, examples of which are on the FHWA website. The standards published by the FHWA show abutment heights up to 24 feet. The Rock County Engineer indicated that he had previously worked with FHWA and was familiar with GRS abutment construction through several projects in Kansas, so he was very comfortable using them for this project.

A high quality granular fill is required for the soil in the GRS system, and a compaction of 95% of maximum dry weight.



**1-2 Cross-Section of GRS Abutment (courtesy of FHWA)**

### *Benefits*

The advantages of the GRS system for abutments include rapid construction and lower costs. Abutments can be constructed in three days, as compared to a week or more for a conventional concrete single line pile bent. The FHWA estimates cost savings of 25-60% on their website.

Along with a shorter duration, construction is also simplified compared to traditional abutment construction. No heavy equipment or specialized labor is needed, and with the exception of heavy rain events during backfilling, the construction operations are less weather dependent than typical abutment construction. The County Engineer noted from his previous experience with GRS Abutments, as long as construction begins at the correct location and elevation, it is fairly simple and he does not anticipate constructability issues.

For the projects that have been built, it has been stated the “bump in the road” which often occurs between the bridge and approach roadway, does not occur for sites with a GRS abutment. Additionally, there are no expansion joints to maintain. Since the ends of the beams are buried, they are often coated with bitumen for protection from moisture and corrosion.

### *Limitations*

A geotechnical investigation is required similar to other bridges to verify the subgrade can support the GRS system, and to design for adequate safety factors for global stability and sliding. The required bearing pressure capacity of the subgrade is 4,000 psf. The FHWA also recommends the bridge span be limited to 140 feet, to limit the reaction and resulting bearing pressure on the GRS system. There is also a limit to the abutment height that is generally controlled by what has been successfully used elsewhere, which is currently about 24 feet.

The Rock County project is a county road over a railroad. Projects elsewhere in the US have used the GRS system for stream crossings. The scour potential of the abutment structure for this system is a concern. Streams with flood potential, rapid flows, and locations that could be inundated would not be good candidates. Where water is present, the flow would need to be negligible, such as a channel between lakes, for the system to be considered.

GRS systems for bridge abutments are a new system for Minnesota designers and contractors. Additional time is needed for designers to become familiar with the methodology and details. Contractors also need to learn the construction methods and system.

### *Cost*

The Rock County project has not yet been let, so there is no project-specific cost information available. As noted above, the FHWA estimates cost savings of 25-60% on their website.

### *Potential Actions to Promote further Implementation*

Without the benefit of a completed project, it is too early to offer recommendations for future implementation. Sharing the results of the Rock County pilot project would be valuable for a venue such as the 2013 County Engineers annual meeting. Following successful completion of this project, the County Engineer noted that he will be trying the GRS technology under the approaches of the bridges they construct using conventional methods to take advantage of the benefit that the GRS can eliminate the settlement that causes a bump at the ends of the bridge.

To prepare local contractors for the upcoming project, the County Engineer has referred potential bidders to the FHWA website to access the YouTube video depicting GRS construction.

### **Innovation 2: Precast Inverted Tee Slab Span Bridges (General)**

#### *Description*

In 2005, MnDOT developed a new precast system for slab span bridges based on a similar section that was in use in France. The 2004 AASHTO and FHWA scanning tour of Prefabricated Bridge Elements and Systems identified this concept as a technology for potential use in the United States. MnDOT involved local fabricators in developing the standards for the precast inverted tee section and the first bridges were built in 2005. As of 2011, MnDOT has constructed eleven bridges using this section, with several additional bridges planned. Currently, Chisago and Scott Counties each have a bridge designed using this section and awaiting letting. See Appendix A2 for an example GP&E sheet and additional photos of this technique.

The prestressed inverted tee sections are placed side by side, providing both a structural beam as well as the bottom form for the composite deck pour. Photo 2-1 shows sections being placed for one of the structures. A reinforcing cage is set in the joint area between sections and cast-in-place (CIP) concrete is placed over the top of the sections, filling the joint and forming the roadway surface. The reinforced joints provide load transfer between sections, enabling the entire system to act as a solid slab span. Photo A2-2 in the Appendix shows the joint area with the drop in reinforcing cage in place, awaiting the CIP concrete pour.



**2-1 Setting precast inverted tees for MnDOT bridge**

The University of Minnesota has conducted extensive research on the inverted tee section, instrumenting bridges in the field and conducting load tests. Additionally, fatigue testing of the sections has been conducted in the Structures Laboratory at the University to assist MnDOT in confirming the durability and composite behavior, and provide data to improve the design. The section is capable of spanning to approximately 60 feet and can be used in continuous multiple span jointless bridges or as a single span.

The bridges currently planned on the county system are:

- Chisago County: County Road 57 over Goose Creek – Bridge No. 13521
- Scott County: CSAH 6 over unnamed stream west of Belle Plaine – Bridge No. 70548

Both bridges have been designed as of March 2012, and are awaiting letting. The Chisago County bridge is a 45-foot single span, the Scott County bridge is a 35-foot span.

### *Benefits*

The precast inverted tee was developed for shorter span bridges as an alternative to CIP slab span structures. The inverted tee provides the same shallow depth of structure and reliable service as a slab span. The benefits of inverted tee include:

- No falsework required: The inverted tee also acts as the bottom form, eliminating the piling and falsework that is typically placed in the stream bed to shore up the forms for a CIP slab span until the concrete has cured.
- Rapid Construction: MnDOT was able to shorten the construction time at a bridge site to 20 days for one of the three-span bridges built, demonstrating the time savings the system provides when acceleration is desired. The elimination of falsework and form construction,



and the reduced amount of reinforcing placed in the field allow significant time savings when compared to the two- to three-month time needed for a CIP slab. MnDOT has estimated typical savings of 20-40% in construction time can be expected.

- Environmental: The falsework required between piers for a CIP slab span is eliminated, reducing the disturbance to the area below the bridge.
- Shallow Depth of Structure: The precast tee structure depth is approximately 2 feet, similar to a CIP slab span bridge. The shallow depth is well suited for replacing older structures such as timber slabs or concrete bridges without requiring a raise in the existing roadway profile.
- Traffic and Worker Safety: For projects where traffic is maintained using staged construction, the reduced construction time also offers the safety benefit of less time in the work zone for construction workers and motorists. If traffic is instead detoured, the safety benefit of a shorter duration in detour time is realized.

### *Limitations*

The design of the inverted tee, particularly for multiple spans, is more complex than a CIP slab span. The designer will need to become familiar with the design considerations for the composite precast and cast-in-place superstructure. There is a learning curve for those new to these designs, and additional design time and the associated costs for the initial designs.

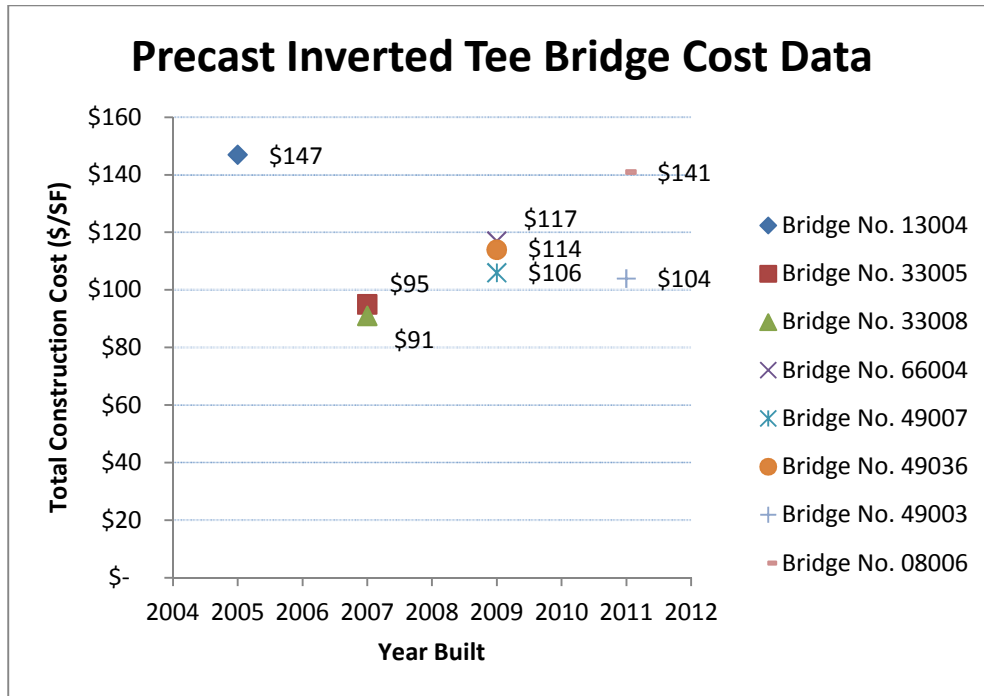
Multiple contractors have built the eleven bridges currently on the MnDOT system and the construction has proceeded similar to other bridges. Placement of the inverted tees on the job site does require crane capacity similar to that required for prestressed beam bridges. A CIP slab or timber slab bridge typically would not require the same lifting capacity. However, the same contractors who build bridges on the local road system will be able to construct the inverted tee.

As more structures have been built, an additional prestressed fabricator has been bidding the projects, providing competition among the suppliers. Currently both County Materials and Cretex Concrete Products have precast forms for these sections.

### *Cost*

Typical of most forms of new construction, the first bridges built using the inverted tees were higher than the average square foot cost of similar Minnesota bridges. In 2005, the first structure cost \$147 per square foot of deck. The higher initial cost is due to factors such as the risk a contractor perceives in constructing a new system, and recovering a significant portion of the investment in new forms for precasting.

Since the introduction in 2005, the cost has declined on the subsequent jobs as the construction methods became known and competition increased among precast beam suppliers. The graph below shows the square foot cost for the bridges from MnDOT records. In a 2010 article for ASPIRE magazine, MnDOT estimated the cost of the precast tee was about 10-15% greater than CIP slabs.



*Potential Actions to Promote further Implementation*

Similar to the GRS abutment project in 2012, the results of the inverted tee projects for Scott and Chisago Counties could be shared at the 2013 County Engineers meeting.

**Innovation 3: Precast Box Beams and Sheet Pile Abutments – Blue Earth County (Bridges 07586, 07593 and 07547)**

*Description*

Blue Earth County has constructed three bridges, 07586 over Little Cobb River and 07593 and 07547 over Big Cobb River, that consist of an adjacent precast box beam superstructure supported on sheet pile abutments. Additionally, a two-line open metal railing was used for the bridge barriers. This design is similar to bridges used in New York for low-volume roads, and was identified as having potential for use in Minnesota during a scanning tour to New York that the Blue Earth County Engineer attended.

Bridges 07593 and 07547 were both constructed with bituminous overlays over waterproofing membranes at the joints, while Bridge 07586 was built with a 5-inch composite CIP reinforced concrete deck due to the higher ADT on CR 168. In 2012, the County is planning to construct two more bridges with ADTs in the 3,000-4,000 range that will use precast adjacent box beams with a 6" reinforced concrete composite/non-composite deck. See Appendix A3 for example details and additional photos of this technique.



**3-1 Blue Earth County Bridge 07547 Precast Box Beams and Sheet Pile Abutments**



**3-2 Blue Earth County Bridge 07593 Sheet Pile Abutment**

## *Benefits*

**Accelerated Construction:** The box beam superstructure can be completed in approximately one week with the beams being set in one day, the keyways grouted over the following several days with 48 hours of cure time, followed by transverse post tensioning. Alternatively, a CIP concrete slab superstructure would require 4-5 weeks to construct, and a prestressed concrete girder and cast-in-place deck bridge would require 2-3 weeks to construct. Additionally, the construction duration for the sheet pile abutments is several days shorter than that for conventional CIP concrete abutments.

**Ability to Withstand Inundation:** The design engineer for Bridges 07547 and 07593 noted that this superstructure type was chosen in part due to its ability to withstand inundation and that it does not necessarily require the freeboard that other structure types do. This makes it an attractive alternative where site constraints dictate a shallow structure type or do not allow for a grade raise.

**Resistance to Approach Fill Loss:** The sheet piles at Bridges 07547 and 07593 extend farther below grade than a single-line pile abutment would, which will help to prevent piping of the fill behind the abutment during flood events. Such piping has been an issue where high water pulls the fill out from behind the abutments, creating a void in the backfill, which ultimately results in settlement of the approaches.

**Temperature Independence:** With the exception of grouting the post tensioning ducts and placement of the overlay, there is nothing temperature sensitive about constructing this bridge type. By eliminating the need for large quantities of CIP concrete, construction of this bridge type can take place over a longer construction season than the alternative bridge types. The possible exception would be bridges that include a CIP reinforced concrete deck on the box beams.

**Shallow Structure Depth and Increased Safety During Construction:** The adjacent box beam offers these benefits similar to the precast inverted tee discussed in Innovation 2.

The benefits of the open rail are discussed in Innovation 5 – Crash Tested Open Metal Railing.

## *Limitations*

Grouting the keyways between beams was a bit problematic during construction of the three bridges, but the results appeared to have improved from one to the next. The contractors tried several methods to address this including using a hangar rod to tighten plywood against the underside of the box as well as shoring the plywood to the underside with support from the ground.

During construction of the first bridge, several of the gaskets that seal the transverse post tensioning ducts shifted out of alignment resulting in grout leakage.

## Cost

Currently, the cost per square foot for the precast box beam bridges is higher than that for alternative bridge types, but cost reductions can be expected with increased use. The following figures were obtained from the MnDOT State Aid Bridge Office 2011 Calendar Year Report:

- Bridge 07547 \$184.54/SF
- Bridge 07593 \$178.93/SF
- Prestressed Girder Average (41 bridges) \$118.83/SF
- Concrete Slab Average (13 bridges) \$109.17/SF

Several factors contributed to the increased cost for this bridge type. While the beam shapes are based on standard shapes used elsewhere, use of precast box beam sections is new to Minnesota bridge construction, and the beams were cast at Wisconsin and Iowa plants where box beam forms are available. Additionally, prior to these bridges, Minnesota contractors had not erected a structure using precast box beams on sheet pile abutments, which also contributed to the increased cost. However, it is promising to note that these projects had multiple bidders, which indicates interest from the contracting community.

Based on the interview with the design engineer for Bridges 07547 and 07593, it is also important to note that while the cost per square foot was higher for these bridges, the structure type was more competitive on an overall project cost basis. All three bridges were simple spans with span lengths of 66-70 feet. A concrete slab structure would have required three spans to avoid a pier at mid stream, and the bridge would likely have been lengthened to balance the spans. However, the design engineer for Bridges 07547 and 07593 indicated that the County Engineer was not interested in having piers at the edges of the stream due to issues with debris accumulation. The other alternative would have been prestressed girders, which are more susceptible to complications caused by inundation and would have required a grade raise to provide adequate freeboard. The grade raise would have increased the bridge length, increased the earthwork and roadway costs, and potentially required additional right-of-way acquisitions. For these reasons, the design engineer noted that the precast box beam bridge had a comparable overall project cost at Bridge 07593 and a lower overall project cost at Bridge 07547.

### *Potential Actions to Promote further Implementation*

Following construction of Bridge 07586, several modifications were made to the designs of Bridges 07593 and 07547 including the following:

Sheetpile Abutment Detail: The first bridge used the sheetpile abutments as the bearing support for the superstructure, which resulted in excessive sheet pile lengths, difficulty establishing the bearing capacity in the field, and extensive cutting and grinding the top of the sheeting to provide a level bearing surface. At the subsequent locations, the superstructure was supported on a steel wide flange welded to the top of a single row of steel pipe piles and the sheeting was installed behind the pipe piles to retain the approach embankment. In addition to the reduced sheetpile quantity, this detailing was significantly easier to construct.

Metal Railing Attachment: The metal railing at Bridge 07586 was extended off the bridge for the transition to the guard rail. In future applications, the County Engineer believes that a conventional plate beam guardrail transition connected to the metal railing would be less costly.

#### **Innovation 4: MSE Wall With Single Line Pile Abutment – Steele County (Bridge 74551)**

##### *Description*

In 2011, Steele County constructed a bridge that utilizes integral abutments on single rows of piles behind MSE walls. While none of the individual components of this abutment type is unique, their use in combination is innovative and unique on Minnesota’s local road system. Bridge 74551 is located on CSAH 7 over the DM&E railroad in Owatonna. It is a single span prestressed girder structure with a span length of 90’-0” from bearing to bearing and a vertical clearance of 23’-4” over the railroad. The superstructure consists of a CIP deck on MN45 beams. Due to a highly compressible clay layer at the project site, the embankments were surcharged for approximately four months prior to abutment construction. In addition to surcharging the embankments, MnDOT installed instrumentation on the piles to monitor settlement of the embankment and the corresponding down drag load on the piles. The results of this monitoring will ultimately be combined with that done at other sites to develop improved MnDOT procedures for drag load design. See Appendix A4 for an example GP&E sheet and additional photos of this technique.



**4-1 Steele County Bridge 74551**

##### *Benefits*

This abutment type allowed for a bridge that was approximately 50% of the length of the 3-span prestressed I-girder alternative that would have been required to meet all of the railroad clearance requests within the federal participation guidelines. The 3-span structure would have had sideslopes in

the end spans with shorter abutments. Combined with the fact that the MSE abutment type uses less concrete and less piling than a typical cast-in-place abutment, the overall cost of the bridge was approximately 25% less than the alternative structure. Another alternative would have been a single span bridge with tall CIP abutments, but that would also have been a more expensive structure.

A benefit of this structure type with regard to future maintenance is the absence of expansion joints on the bridge.

In the case of compressible soils, this abutment type generally results in less settlement than a spread footing would, and MSE walls are more tolerant of settlement than other wall types.

#### *Limitations*

Since this abutment type is not widely used on the local road system, the designer indicated that there was additional time and effort involved in the structure type selection phase compared to a more conventional structure configuration to establish the viability of this abutment type at this site. Documenting successful use of this abutment type elsewhere was key to gaining approval.

Coordination between the MSE wall supplier and contractor is important to avoid construction delays, and can be dependent on the experience and expertise of both parties. Ideally, the contractor would have prior experience with MSE wall installation, and the MSE wall supplier would provide an experienced project liaison.

This abutment type is sensitive to pile alignment, which was an issue on this project; so for future use, the design engineer suggested paying particular attention to those details and including more stringent plan notes to that effect. The designer also suggests, for future projects, allowing enough space between the front face of the abutment and the MSE wall to allow for more construction tolerance.

Additionally, MSE systems generally should not be used where buried utilities may need to be installed in the future. Disturbance of the reinforcing straps within the MSE backfill can threaten the structural integrity of the wall system.

#### *Cost*

According to the designer, the cost of this bridge was approximately 25% lower than what the alternative 3-span structure would have cost.

#### *Potential Actions to Promote further Implementation*

For future use of this abutment type, it is suggested to define standards and limitations with regard to their use similar to when MnDOT began using integral abutments on the local road system. For example, define the maximum allowable span length, skew angle, etc. for use without special review.



## **Innovation 5: Crash Tested Open Metal Railing – Polk County (Bridges 60588 and 60559)**

### *Description*

A crash tested open metal railing was utilized for several bridges in Polk County. In 2008, Bridges 60588 and 60559 over Sand Hill River were constructed using this railing based on a New York design identified from a State Aid scanning tour. That railing is shown in the Photo 5-1 and has been crash tested to meet Test Level 4 requirements for use in projects with design speeds over 40 mph. The bridges are prestressed girder spans.



**5-1 Polk County Bridge 60559 Metal Railing**

Additionally, Blue Earth County has installed the metal railing on three projects with precast box beams for the superstructure. The rail post to deck attachment was modified in the latter Blue Earth County projects to connect the railpost to the side of the precast box girder superstructure. The Blue Earth County projects used a two line metal railing, which meets the Test Level 2 standards for speeds of 40 mph or less.





**5-2 Blue Earth County Bridge 07593**

A modified version also meeting Test Level 4 for higher speed designs was later used in 2009-10 for construction of Bridge 60561, CSAH 9 over the Red River in Polk County, east of Thompson, North Dakota. See Appendix A5 for example details and additional photos of the metal railings.

#### *Benefits*

The Polk County Engineer noted the use of the metal railing design bolted directly to the deck allows the elimination of deck drains. The open base allows an unobstructed flow of water off the bridge for low volume roads. That avoids several issues such as the need to have sufficient grade to ensure the drainage will carry off the ends of the bridge or to a deck drain without ponding, and debris blockage of the deck drain. With the open railing design, light snow can be pushed through the railing depending on the snow event. Blowing snow also does not accumulate against the open rail and at times allows the deck to be blown clean.

Construction is more rapid than concrete rails since no forming, placing of reinforcement, or pouring and curing is required. For projects where accelerated construction is desired the railing reduces construction time.

The MnDOT State Aid Bridge Engineer further noted the open design increases visibility of traffic when intersecting roadways are present at the end of a bridge.

For Bridge 60561 over the Red River, the open railing design allows motorists a view of the river and valley, a more attractive design than a Type F solid concrete barrier. The metal rail can be painted if a color is desired for aesthetics. Issues such as uneven concrete finishing are avoided.

#### *Limitations*

Overall maintenance cost could be higher if traffic hits are incurred compared to a concrete rail. The bridges in Polk and Blue Earth County are on a tangent section of roadway, so the chance of vehicles

losing control and striking a rail are considerably less than a curved alignment. Severe traffic hits however may require replacement of metal members. The bolted connections of railing to post and post to deck allow for replacement of damaged pieces as long as the cast in place post anchorage remains intact.

For higher traffic sites such as Bridge 60561, with 1,100 ADT, a more substantial 2-line rail with a concrete curb was used. This allows motorists a more open view of the Red River Valley as they cross the structure than a solid concrete rail. The design with the curb requires water be carried to either the end of the bridge or deck drains, negating the drainage advantage.

#### *Cost*

Costs of the metal rails are currently higher than concrete railings. The MnDOT State Aid Bridge Engineer provided cost information for the Polk County projects. The metal rails for Bridges 60588 and 60559 were bid at \$220 per LF vs. \$70 per LF typically bid for a concrete railing. With increased use the metal design would become more competitive as fabricators and contractors are accustomed to supplying and installing the rail. In New York State, where the metal railing system use is common, costs are actually less than concrete barriers.

#### *Potential Actions to Promote further Implementation*

The Polk County Engineer stated he would use the railing again for projects similar to the Sand Hill River bridges or the Bridge 60561 replacement over the Red River. The rail was easily installed by the contractor and went quickly, saving the time to form, pour and cure a concrete railing. Blue Earth County also recognized the benefits of the railing for drainage and ease of installation and included them for their precast box girder bridges. As more counties and designers become aware of the railing use in Minnesota it has application on additional projects. While the cost is higher, the metal railing only accounts for approximately 6% of total bridge cost.

### **Innovation 6: Large Precast Box Culverts – Aitkin County (Bridge 01J31) and Three-Sided Structures (General)**

#### *Description*

Aitkin County replaced an existing bridge with a large precast box culvert structure for Bridge No 01J31, County Road 73 over the Sandy River (Co. Ditch #42) near McGregor, Minnesota. The structure is a 20 feet wide and 8 feet high (20'x8') which exceeds the maximum span of 16 feet covered by the MnDOT standard culvert designs tables. An engineer was retained to design the reinforcing and modify the MnDOT standards, and the culvert was constructed in 2011.



**6-1 Aitkin County Bridge 01J31**

### *Benefits*

A set of twin boxes was not desired at this location, so a large single box structure was chosen with the intent of maintaining the full waterway opening across the entire width of the box.

From conducting bridge inspections for a number of years, the County Engineer noted that double and triple box culvert installation often did not function hydraulically as envisioned. Quite frequently some amount of channel change had been required during construction to align or modify the channel in an attempt to direct the flow through the double/triple boxes. The stream however would soon migrate back to its natural flow and primarily utilize only one of the culvert barrels. The second or third box would silt in with sediment or debris, no longer providing the full hydraulic cross section.

After observing this tendency for a multiple barrel structure to partially silt in, the county developed a preference for a single span structure where feasible.

During the design phase the size of the boxes was reviewed as noted above for constructability. The county and designer believed local contractors would not have any issues building the culvert. This assessment was confirmed by the fact eight bidders competed for the project, the typical small contractors that bid on other projects in Aitkin County. No company expressed concerns to the County regarding the box size or constructability.

Maintenance will be the same as for all precast box culvert structures, with the exception less debris and sediment collection and corresponding cleanout should be needed in comparison to a multi-barrel box. In general, the precast box culverts have required very little maintenance for structural items.

The County Engineer expects benefits over the life of the structure due to less maintenance required by county forces to keep the culvert free of debris or sediment. For future projects he will consider use of larger boxes and is using an 18-foot box for a project in 2012.

#### *Limitations*

For some sites, the access and placement of larger box sections may be an issue. Larger crane capacity is required for placement of the sections. Additionally, shipping weight and size of boxes could be an issue for trucking. The county explored those issues for this project and confirmed that the site allowed adequate access for equipment and precasters could ship to the site.

#### *Cost*

The bid price for the 20'x8' single box was \$1,540 per LF, and end sections were bid at \$16,000 each. For comparison purposes, a recent Aitkin County bid for a 14'x6' went for \$850 per LF and \$10,000 per end section. Twin lines of 14'x6' would approximate a single 20'x8'. Although that is just one comparison, the County believes that the cost of the large single box is comparable to twin boxes of equivalent area.

The project did come in close to the Engineer's estimate. Estimated cost was \$157,000 and actual low bid was \$166,000. In summary, the County did not see a premium paid for the innovation.

A significant cost for these structures is in the end sections. In lieu of the wide end sections for a 20-foot box, the use of precast wingwalls should be considered. That decision will be site specific and the roadway geometry must be considered.

#### *Potential Actions to Promote further Implementation*

At the County Engineers Bridge Subcommittee meeting in January 2012, it was requested that the use of Large Culverts be added to the report so County Engineers are aware they can go beyond the limits of the standard box culvert designs in the MnDOT tables. It does require retaining an engineer to do some design, to extend the design for the longer width. Routine culvert spans within the limits of the standard tables can be designed using MnDOT published standards.

The fact that these longer spans do require hiring an engineer could be a limitation to expanding usage of longer spans. If other counties also choose to use longer spans and the demand develops, then the standard MnDOT culvert tables should be expanded once again, beyond the current 16-foot span. The tables have been expanded in the past for these reasons.

#### *Three-Sided Structures*

In addition to large precast box structures, there has been an increased use of three-sided structures for local roads and the MnDOT Trunk Highway system. Three-sided structures are precast but do not have a bottom slab. The legs bear on a footing that is cast in place on the site.

Spans for the three-sided structures can go up to 60 feet, however the common spans are typically 28 to 42 feet. Similar to box culverts, the structure is built from a series of precast sections that are sized for shipping and lifting.

Benefits of three-sided structures include the fact it is a low maintenance structure being a culvert, and the stream bottom is undisturbed and maintains a natural bottom. The natural bottom is preferred in streams where there is concern for fish migration or habitat.

Limitations include the fact that scour susceptible sites can require a pile foundation, which increases the cost of the structure significantly. The roadway barrier on top of the structure is typically a moment slab, where the railing is anchored into the pavement to prevent the railing from overturning from traffic hits. The three-sided structure is not designed to anchor the barrier railing directly.

Cost are usually higher than precast box culverts, so use of a three-sided structure is typically at sites where the open bottom is needed or the arch-like appearance is desired for aesthetics .

Photo 6-2 shows a three-sided structure under construction in Morrison County; see Appendix A6 for an example GP&E sheet for this technology as well as photos of MnDOT Bridge 16X03.



**6-2 Morrison County Bridge 49J44**

### **Recommendations for Research or Implementation**

During the interviews with County Engineers, the MnDOT State Aid Bridge Engineer, and FHWA, it was asked if they had any recommendations for research regarding innovative construction or technologies that should be considered for implementation. It was noted by a county engineer that Minnesota counties, MnDOT and industry have made good progress in the last 20 years and innovations have been made available. The counties have done a good job expanding usage and implementing new designs, and it is believed that trend will continue in the future.

The following is a listing of the ideas suggested or comments offered in response to the question:

- For the local road system, cost is probably a more significant driver than Accelerated Bridge Construction (ABC). Costs for any system need to be driven down to gain increased use. That can occur as fabricators become accustomed to casting new sections and contractors become familiar with the new method of construction.
- Construction costs may also be reduced by having competing structure types, however the design costs of developing alternative systems is a barrier unless standard plans are available similar to the box culvert standards.
- When successful projects are completed, those need to be marketed to county engineers and bridge owners so they are aware of the benefits and the limitations. Success in one county increases the likelihood another will try the innovation.
- The scanning tours have helped to make county engineers, State Aid, and designers aware of the methods used in other states. The upcoming scan to Iowa in 2012 includes ultra high performance concrete, GRS abutments, and precast slab span panels. That information will be shared with Minnesota county engineers and provides an opportunity to try additional technologies.
- Some innovations, such as precast box culverts or precast inverted tees, will benefit from development of standard designs. This can aid designers and owners in specifying the bridge type, reducing the design effort. Extension of the precast box culvert standards from the current 16 foot to 18 or 20 foot was mentioned.
- Similar to the previous comment, as an aid to designers of county bridges, a design example of a multiple span inverted tee could be developed. This would provide them the method MnDOT has used to consider the continuity and time dependent behavior of the sections. This would lessen the learning curve in implementing inverted tee designs and assist designers for local roads since the design budgets for these projects are limited.
- Consider investigating if the span of the MnDOT inverted tee could be extended further than the 55-60-foot limit by section modifications such as; 1) increasing the web depth and introducing a void similar to a box beam to reduce the weight, and 2) adding additional post tensioning with the added depth.

### **Opportunities for Federal Funding of Innovative Designs**

The typical federal funding sources available for any local road bridge project are of course available for projects involving innovations. The following federal funding programs are additional programs specifically developed to encourage innovations and were discussed in an interview with the FHWA Minnesota Division Bridge Engineer:

#### *Innovative Bridge Research and Deployment (IBRD)*

The IBRD program provides federal funds for projects that demonstrate innovative accelerated bridge design and construction technology, and the application of innovative material technology for bridges. The program was funded within the SAFETEA-LU federal transportation act and has been extended via

the Surface Transportation Extension Act for Fiscal Year 2012. Solicitations for IBRD and the Highways for Life Program most recently occurred in January of 2012. The FHWA solicits projects from the states, reviews applications based on conformance with the program goals, and selects projects according to available funding. The IBRD program funding amounts vary yearly with congressional appropriations and have ranged from \$4 to \$13 million. Individual projects selected vary in funding amounts from as low as \$30,000 to \$400,000 or greater.

The goals of the program include development of cost-effective highway bridge applications and construction techniques, development of engineering design criteria for innovative products, materials and structural systems, and effective transfer of resulting information and technology. The FHWA website contains more detailed information.

Several Minnesota projects have received IBRD funding in past years. For the local road system, the Rock County Bridge 67564 received \$350,000 for the use of Geosynthetic reinforced soil (GRS) abutments. That project is described within the synthesis report and will be constructed in 2012.

It is unknown if the next federal transportation funding act will continue the IBRD program. The programs within each act will be determined by Congress.

#### *Highways for Life*

The Highways for Life program has also been a source of funds for innovative construction. MnDOT's replacement of the Maryland Avenue Bridge over I-35E in St. Paul was partially funded by this program. Construction of that project will begin in May of 2012 and includes the first use of self propelled modular transporters (SPMT). The bridge superstructure (beams and deck) will be built off the I-35E alignment on falsework and moved over a weekend in place on the new piers by the SPMTs. This eliminates beam erection and deck construction over the active interstate.

MnDOT has also applied for Highways for Life funding for a TH 61 project to build a bridge and slide it into position in the Rochester District and for construction using full depth precast concrete deck panels in the Duluth District.

The Highways for Life program projects are typically larger projects than IBRD in terms of funding, often several million in cost. For local road use, the IBRD program is a more likely source of funding.

Similar to IBRD, it is unknown if the next transportation act will continue the Highways for Life program.

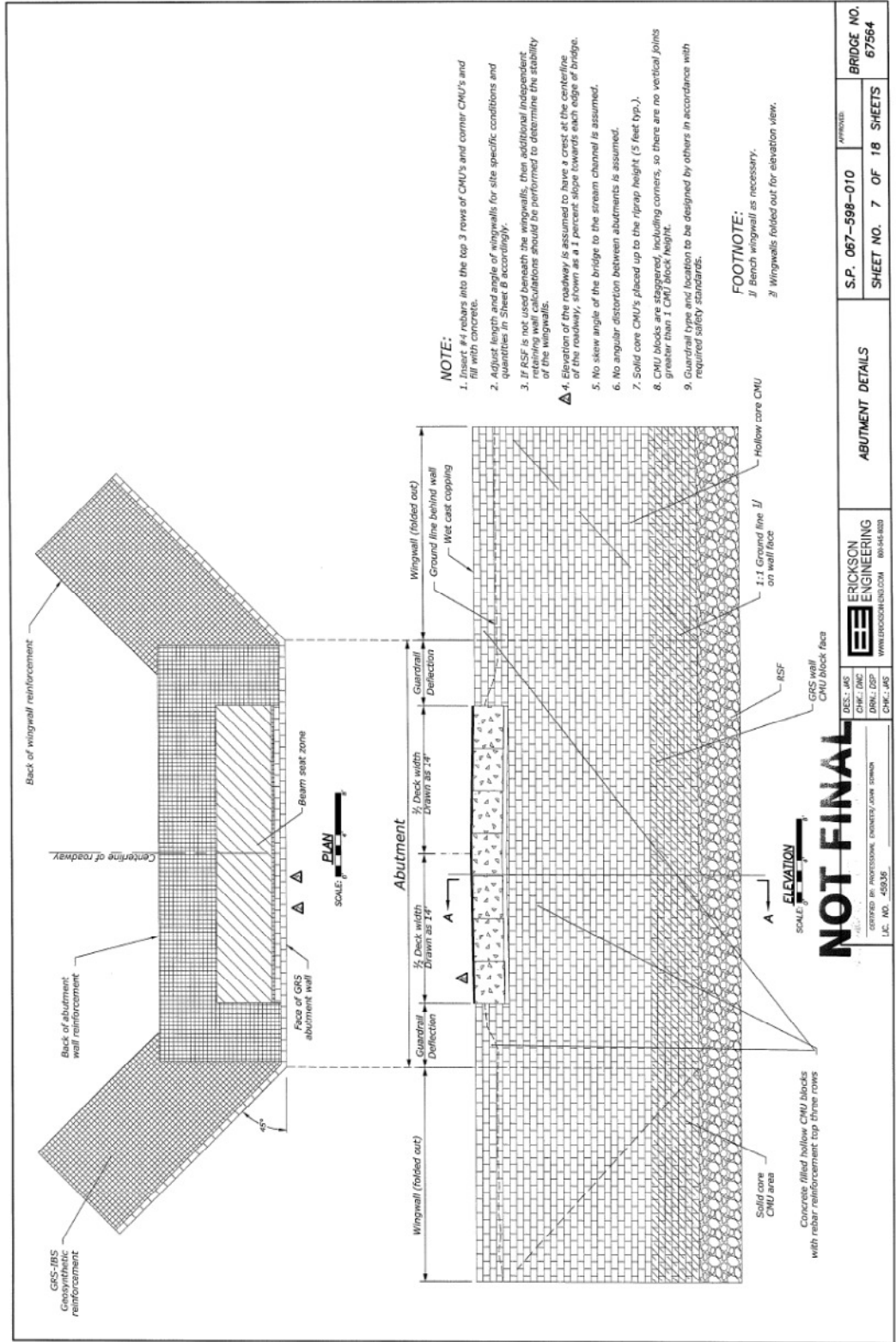
## Innovative Bridge Techniques Summary

See the individual technology sections for more detailed discussion.

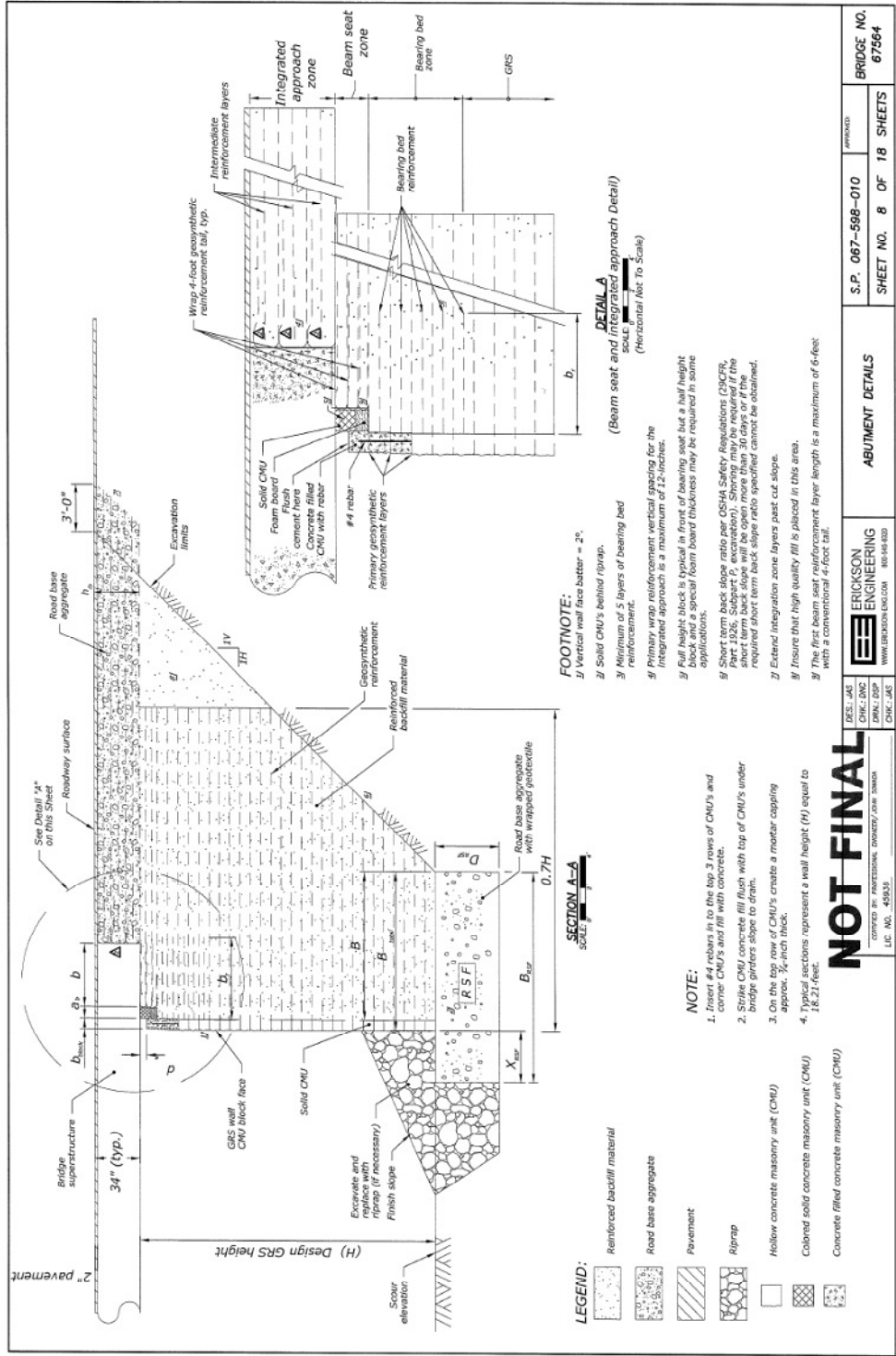
<i>Technique Description</i>	<i>Considerations for Use</i>	<i>Cost Information</i>
Geosynthetic-Reinforced Soil (GRS) Abutments	<ul style="list-style-type: none"> <li>• Bridge length <math>\leq</math> 140 ft</li> <li>• Abutment height <math>\leq</math> 24 ft</li> <li>• Soil bearing pressure capacity <math>\geq</math> 4,000 psf</li> <li>• Good for overpasses of roadway or RR</li> <li>• Not recommended at waterway crossings with potential for scour</li> <li>• Allows more rapid construction than CIP</li> </ul>	Cost savings of 25-60% per FHWA est.
Precast Inverted Tee Slab Span Bridges	<ul style="list-style-type: none"> <li>• Span length <math>\leq</math> 60 ft</li> <li>• Shallow superstructure locations – good replacement for timber or CIP slabs</li> <li>• Rapid construction, est. construction time savings of 20-40%</li> <li>• More complex design than CIP slab, learning curve for new designers</li> </ul>	Cost approx. 10-15% higher than CIP slab per MnDOT est. Cost has declined as more structures have been built.
Precast Box Beams and Sheet Pile Abutments	<ul style="list-style-type: none"> <li>• Rapid construction with limited temperature dependence</li> <li>• Shallow superstructure, good for locations subject to inundation</li> <li>• Learning curve with first-time contractors</li> <li>• Precast box beams and sheet pile abutments are separate innovations that can be used in combination or with other types of abutments and superstructures.</li> <li>• Max. span length for 27" box beam is approx. 90 ft.</li> </ul>	Cost/SF approx. 50% higher than prestressed beam or CIP slab per MnDOT State Aid Bridge Office data, but can be offset by shorter bridge and less grading (see p.12).
MSE Wall with Single Line Pile Abutment	<ul style="list-style-type: none"> <li>• Good for overpasses of roadway or RR</li> <li>• Can provide shorter bridge than with sideslopes underneath</li> <li>• Can provide lower cost bridge than with tall CIP abutments</li> <li>• No expansion joints on bridge</li> </ul>	Cost approx. 25% lower than alternative structure type per design engineer
Crash Tested Open Metal Railing	<ul style="list-style-type: none"> <li>• Three-line rail crash tested to meet TL-4 (design speed <math>&gt;</math> 40 mph)</li> <li>• Two-line rail crash tested to meet TL-2 (design speed <math>\leq</math> 40 mph)</li> <li>• Open base allows unobstructed water flow off bridge deck</li> <li>• Increased visibility for traffic</li> <li>• Alt. installation with concrete curb</li> </ul>	Currently higher cost than concrete railing
Large Precast Box Culverts	<ul style="list-style-type: none"> <li>• Aitkin Co. used 20'x8'</li> <li>• Minimizes silting and debris collection versus twin boxes</li> <li>• Box size did not present additional constructability issues, but may not be feasible for sites with access or trucking limitations</li> </ul>	Aitkin Co. project cost of large single box was comparable to cost of twin boxes with equivalent area
Three-Sided Structures	<ul style="list-style-type: none"> <li>• Common spans of 28-42 ft</li> <li>• Natural stream bottom retained</li> <li>• Requires CIP footings</li> <li>• Scour susceptible sites can require piles</li> </ul>	Typ. higher cost than precast box culverts, so best for sites that require aesthetics or natural bottom







A1-2 GRS Abutment Plan Sheet for Bridge 67564



A1-3 GRS Abutment Detail Sheet for Bridge 67564

**LEGEND:**

- Reinforced backfill material
- Road base aggregate
- Pavement
- Riprap
- Hollow concrete masonry unit (CMU)
- Colored solid concrete masonry unit (CMU)
- Concrete filled concrete masonry unit (CMU)

**NOTE:**

- Insert #4 rebar in to the top 3 rows of CMU's and corner CMU's and fill with concrete.
- Strike CMU concrete fill flush with top of CMU's under bridge girders slope to drain.
- On the top row of CMU's create a mortar capping approx. 1/4 inch thick.
- Typical sections represent a wall height (H) equal to 18.2'-feet.

**FOOTNOTE:**

- Vertical wall face batter = 2°.
- Solid CMU's behind riprap.
- Minimum of 5 layers of bearing bed reinforcement.
- Primary wrap reinforcement vertical spacing for the integrated approach is a maximum of 12-inches.
- Full height block is typical in front of bearing seat but a half height block and a special foam board thickness may be required in some applications.
- Short term back slope ratio per OSHA Safety Regulations (29CFR, Part 1926, Subpart P, excavation). Shoring may be required if the required short term back slope ratio specified cannot be obtained.
- Extend integration zone layers past cut slope.
- Ensure that high quality fill is placed in this area.
- The first beam seat reinforcement layer length is a maximum of 6-feet with a conventional 4-foot tail.

**DETAIL A**  
 (Beam seat and integrated approach detail)  
 SCALE: (Horizontal/Not To Scale)

**DETAIL B**  
 (Beam seat zone)  
 SCALE: (Horizontal/Not To Scale)

**DETAIL C**  
 (Integrated approach detail)  
 SCALE: (Horizontal/Not To Scale)

**DESIGNER:** JAC  
**CHECKER:** JAC  
**DATE:** 08/11/11  
**PROJECT NO.:** 067-598-010  
**BRIDGE NO.:** 67564  
**SHEET NO. 8 OF 18 SHEETS**

**ERICKSON ENGINEERING**  
 WWW.ERICKSON-ENG.COM 800-545-0303

**NOT FINAL**  
 PREPARED BY: APPROXIMATE, DIRECTLY FROM DRAWING



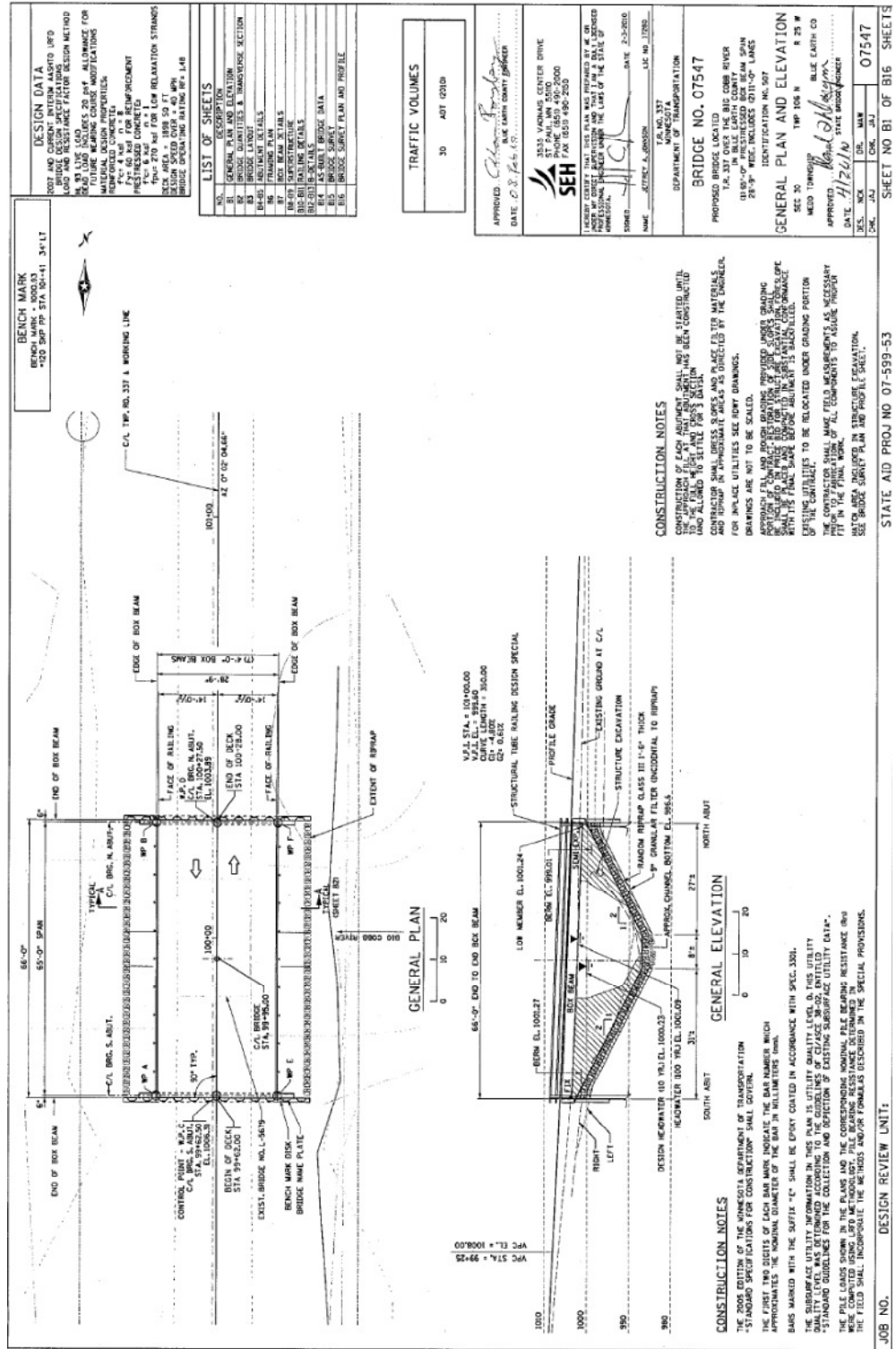


**A2-2 Drop-in reinforcing cage between inverted tee sections**



**A2-3 MnDOT precast inverted tee Bridge 13004**

A3: Precast Box Beams and Sheet Pile Abutments



A3-1 GP&E Sheet for Bridge 07547







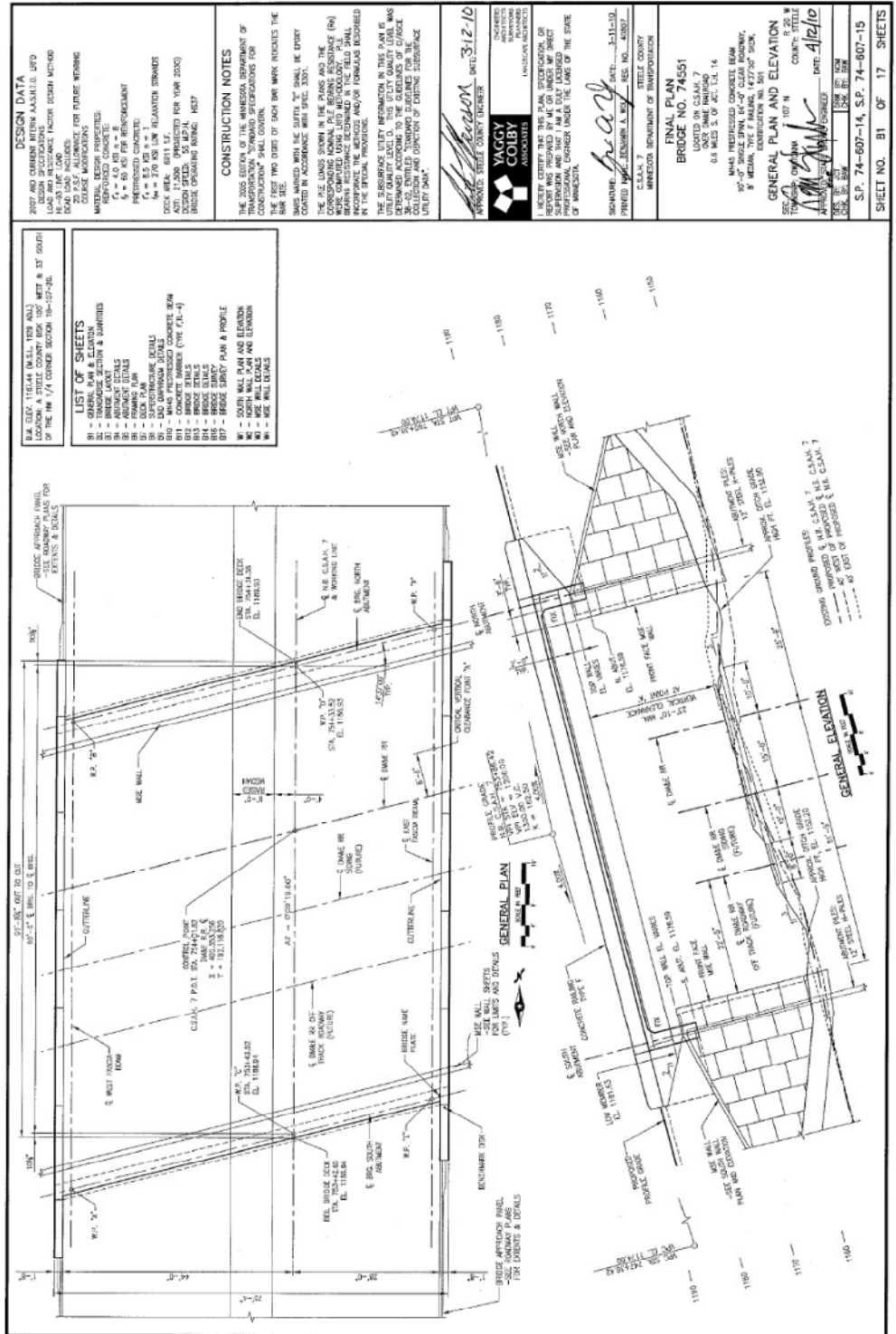
**A3-4 Sheetpile abutment at Bridge 07593**



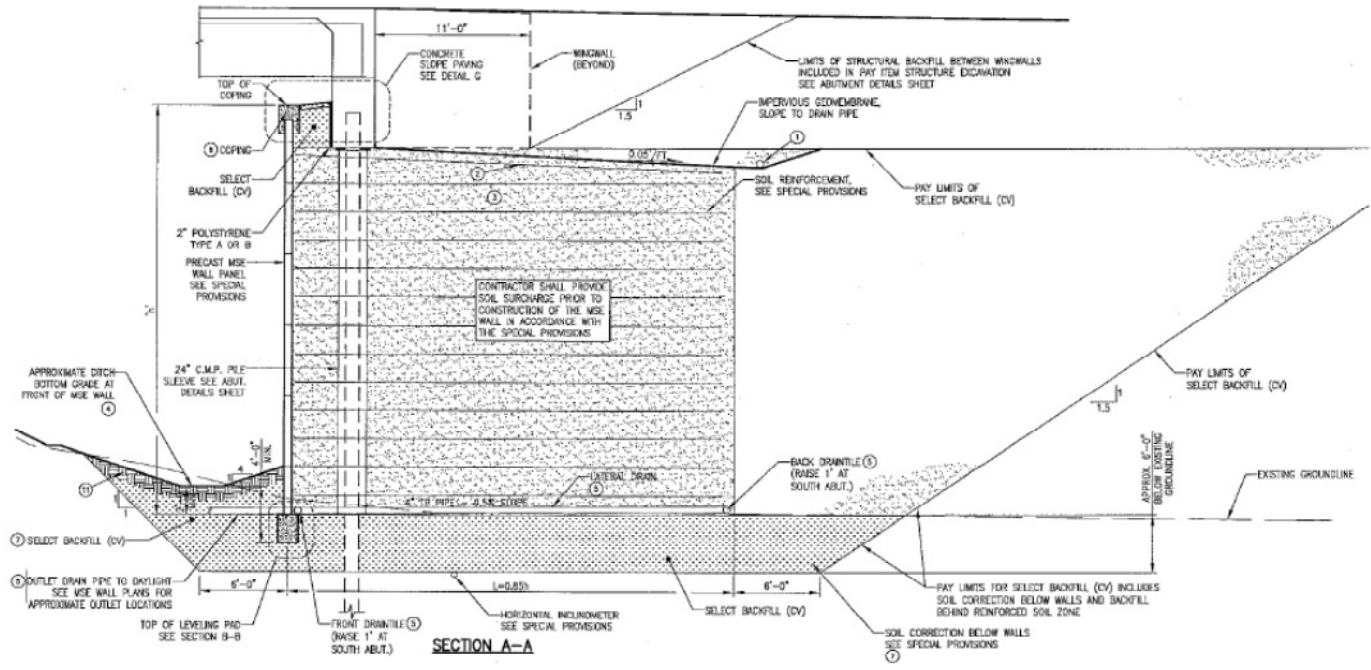
**A3-5 Sheetpile abutment wing at Bridge 07593**



# A4: MSE Wall With Single Line Pile Abutment



A4-1 GP&E Sheet for Bridge 74551



**A4-2 MSE Wall Detail for Bridge 74551**



**A4-3 MSE Wall Construction for Bridge 74551**



**A4-4 MSE Wall and Abutment Piles for Bridge 74551**



**A4-5 MSE Wall and Abutment Piles for Bridge 74551**

A5: Crash Tested Open Metal Railing

**DESIGN DATA**

2004 & CURRENT MINN. S.D.S. 13.0 F.S. BRIDGE DESIGN SPECIFICATIONS LOAD & RESISTANCE FACTOR DESIGN METHOD - LR-50 LIVE LOAD DESIGN METHOD - LR-50 ALLOWANCE FOR FUTURE WEARING COURSE MODIFICATIONS.

MAXIMUM ALLOWABLE DESIGN STRESSES:  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 PRESTRESSED CONCRETE REINFORCEMENT  
 F<sub>u</sub> = 250,000 PSI  
 F<sub>y</sub> = 180,000 PSI  
 STRUCTURAL STEEL:  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 END EMPIRICAL DETAILS  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 PRESTRESSED CONCRETE BEAM  
 F<sub>u</sub> = 250,000 PSI  
 F<sub>y</sub> = 180,000 PSI  
 METAL BRIDGE DETAILS  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 BRIDGE SURVEY - PLAN & PROFILE

DESIGN WIND AREA = 429 SQ. FT.  
 WIND VELOCITY = 110 MPH  
 OPERATING RATING = HS 39  
 PROTECTIVE RATING = HS 39  
 DESIGN SPEED = 55 MPH

**LIST OF SHEETS**

131	GENERAL PLAN & ELEVATION
132	BRIDGE LAYOUT
133	ABUTMENT DETAILS
134	PIER DETAILS
135	SUPERSTRUCTURE DETAILS
136	END EMPIRICAL DETAILS
137	PRESTRESSING DETAILS
138	TUBULAR STEEL RAILING (TYPE W)
139	MISC. BRIDGE DETAILS
140	BRIDGE SURVEY - PLAN & PROFILE

**CONSTRUCTION NOTES**

THE 2005 EDITION OF THE MINNESOTA DEPARTMENT OF TRANSPORTATION STANDARD SPECIFICATIONS FOR CONSTRUCTION SHALL GOVERN.

INDICATE TWO DIGITS OF EACH BAR MARK WITH THE SUFFIX 'E' SHALL BE EPOXY COATED.

THE SURFACE UTILITY INFORMATION IN THIS PLAN IS UTILITY QUALITY LEVEL D. THIS UTILITY INFORMATION IS FOR INFORMATION ONLY. THE GENERALS OF DIVISION 31-02. EXCEPT AS SHOWN OTHERWISE, ALL UTILITIES SHALL BE DELETED OR COVERED SUBSURFACE UTILITY DATA.

BRIDGE SHALL BE VERTICAL DURING THE BRIDGE STREAM LOCATION AT THE TIME OF CONSTRUCTION.

CONSTRUCTION SHALL BE ACCORDING TO THE STANDARD SPECIFICATIONS FOR CONSTRUCTION. THE STANDARDS IS CHANGED IN THE PLANS SHALL BE SUBMITTED TO THE OWNER AND THE PILE LOADS SHOWN IN THE PLANS AND THE CHANGING NOMINAL PILE BEARING RESISTANCE (PILE BEARING RESISTANCE) LIFT METHODOLOGY. PILE BEARING RESISTANCE LIFT METHODOLOGY SHALL BE USED WITHOUT THE SPECIAL PROVISIONS.

**DESIGN DATA**

2004 & CURRENT MINN. S.D.S. 13.0 F.S. BRIDGE DESIGN SPECIFICATIONS LOAD & RESISTANCE FACTOR DESIGN METHOD - LR-50 LIVE LOAD DESIGN METHOD - LR-50 ALLOWANCE FOR FUTURE WEARING COURSE MODIFICATIONS.

MAXIMUM ALLOWABLE DESIGN STRESSES:  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 PRESTRESSED CONCRETE REINFORCEMENT  
 F<sub>u</sub> = 250,000 PSI  
 F<sub>y</sub> = 180,000 PSI  
 STRUCTURAL STEEL:  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 END EMPIRICAL DETAILS  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 PRESTRESSED CONCRETE BEAM  
 F<sub>u</sub> = 250,000 PSI  
 F<sub>y</sub> = 180,000 PSI  
 METAL BRIDGE DETAILS  
 F<sub>u</sub> = 65,000 PSI  
 F<sub>y</sub> = 48,000 PSI  
 BRIDGE SURVEY - PLAN & PROFILE

DESIGN WIND AREA = 429 SQ. FT.  
 WIND VELOCITY = 110 MPH  
 OPERATING RATING = HS 39  
 PROTECTIVE RATING = HS 39  
 DESIGN SPEED = 55 MPH

**LIST OF SHEETS**

131	GENERAL PLAN & ELEVATION
132	BRIDGE LAYOUT
133	ABUTMENT DETAILS
134	PIER DETAILS
135	SUPERSTRUCTURE DETAILS
136	END EMPIRICAL DETAILS
137	PRESTRESSING DETAILS
138	TUBULAR STEEL RAILING (TYPE W)
139	MISC. BRIDGE DETAILS
140	BRIDGE SURVEY - PLAN & PROFILE

**PLAN**  
SCALE: 1" = 40'

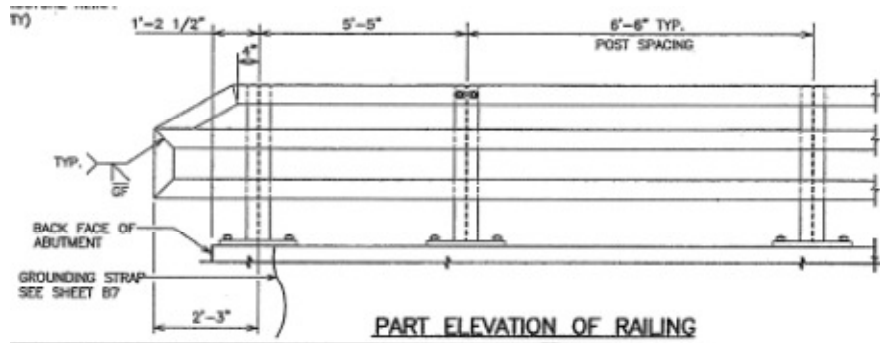
**ELEVATION**  
SCALE: 1" = 10'

**TRANSVERSE SECTION THRU DECK**  
SCALE: 1" = 12"

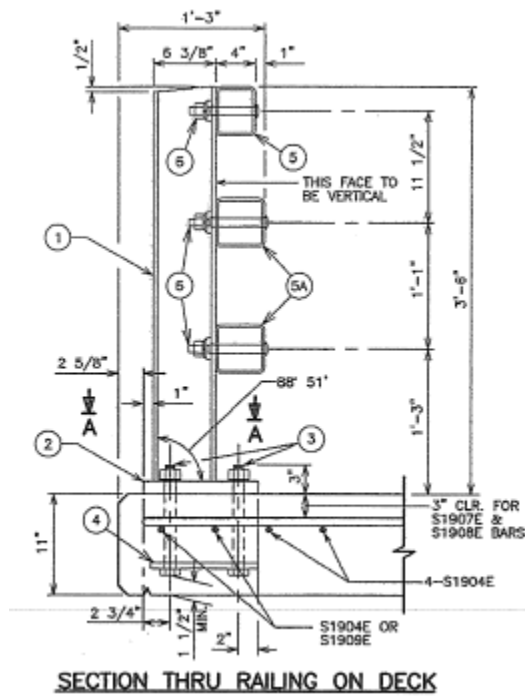
**SCHEDULE OF QUANTITIES FOR THE ENTIRE BRIDGE**

ITEM NO.	DESCRIPTION	UNIT	QUANTITY	UNIT PRICE	TOTAL AMOUNT
2001.001	MADEUPON	EA	2	100.00	200.00
2001.002	MADEUPON	EA	2	100.00	200.00
2001.003	MADEUPON	EA	2	100.00	200.00
2001.004	MADEUPON	EA	2	100.00	200.00
2001.005	MADEUPON	EA	2	100.00	200.00
2001.006	MADEUPON	EA	2	100.00	200.00
2001.007	MADEUPON	EA	2	100.00	200.00
2001.008	MADEUPON	EA	2	100.00	200.00
2001.009	MADEUPON	EA	2	100.00	200.00
2001.010	MADEUPON	EA	2	100.00	200.00
2001.011	MADEUPON	EA	2	100.00	200.00
2001.012	MADEUPON	EA	2	100.00	200.00
2001.013	MADEUPON	EA	2	100.00	200.00
2001.014	MADEUPON	EA	2	100.00	200.00
2001.015	MADEUPON	EA	2	100.00	200.00
2001.016	MADEUPON	EA	2	100.00	200.00
2001.017	MADEUPON	EA	2	100.00	200.00
2001.018	MADEUPON	EA	2	100.00	200.00
2001.019	MADEUPON	EA	2	100.00	200.00
2001.020	MADEUPON	EA	2	100.00	200.00
2001.021	MADEUPON	EA	2	100.00	200.00
2001.022	MADEUPON	EA	2	100.00	200.00
2001.023	MADEUPON	EA	2	100.00	200.00
2001.024	MADEUPON	EA	2	100.00	200.00
2001.025	MADEUPON	EA	2	100.00	200.00
2001.026	MADEUPON	EA	2	100.00	200.00
2001.027	MADEUPON	EA	2	100.00	200.00
2001.028	MADEUPON	EA	2	100.00	200.00
2001.029	MADEUPON	EA	2	100.00	200.00
2001.030	MADEUPON	EA	2	100.00	200.00
2001.031	MADEUPON	EA	2	100.00	200.00
2001.032	MADEUPON	EA	2	100.00	200.00
2001.033	MADEUPON	EA	2	100.00	200.00
2001.034	MADEUPON	EA	2	100.00	200.00
2001.035	MADEUPON	EA	2	100.00	200.00
2001.036	MADEUPON	EA	2	100.00	200.00
2001.037	MADEUPON	EA	2	100.00	200.00
2001.038	MADEUPON	EA	2	100.00	200.00
2001.039	MADEUPON	EA	2	100.00	200.00
2001.040	MADEUPON	EA	2	100.00	200.00
2001.041	MADEUPON	EA	2	100.00	200.00
2001.042	MADEUPON	EA	2	100.00	200.00
2001.043	MADEUPON	EA	2	100.00	200.00
2001.044	MADEUPON	EA	2	100.00	200.00
2001.045	MADEUPON	EA	2	100.00	200.00
2001.046	MADEUPON	EA	2	100.00	200.00
2001.047	MADEUPON	EA	2	100.00	200.00
2001.048	MADEUPON	EA	2	100.00	200.00
2001.049	MADEUPON	EA	2	100.00	200.00
2001.050	MADEUPON	EA	2	100.00	200.00

A5-1 GP&E Sheet for Bridge 60559

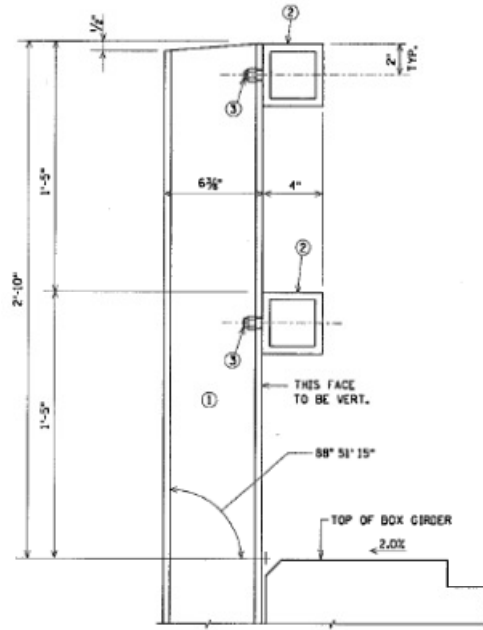


**A5-2 Railing Elevation Detail for Bridge 60559**



**A5-3 Section Thru Railing Detail for Bridge 60559**





SECTION THRU RAILING ON DECK

**A5-4 Section Thru Two-Line Railing on Blue Earth Co. Bridge 07547**



**A5-5 Two-Line Railing Attachment on Blue Earth Co. Bridge 07593**





**A6-2 MnDOT Bridge 16X03 over Kimball Creek; 32-foot span (courtesy of Cretex Concrete Products)**



**A6-3 MnDOT Bridge 16X03 over Kimball Creek; 32-foot span (courtesy of Cretex Concrete Products)**